

# Calculation Models for Ultimate Limit States

Andrew Smith  
Coffey Geotechnics Ltd.



## Objectives

- What is new about EC7 Ultimate Limit State Design?
- How does EC7 ULS design appear?
  - From the point of view of a user
- How can we best use EC7 ULS methods?

## Contents

- What is new in EC7
- Attributes of a new design code
- Some perceptions of EC7
- ULS in EC7 and before
- Use of EC7

## What has changed?

- Much of EC7 is little different from previous practice, e.g.:
  - SLS calculations with partial factors unity
  - SI practice
- The main change is in ULS design:
  - Formalises definition of ULS
  - Terminology:  
action, effect, resistance
  - Use of partial factors

## Attributes of a New Design Code

- Comprehensiveness
- Ease of Use
- Consistency
  - Internal
  - With previous codes
  - With physical reality
- Leads to reliable and economic design

## Some Perceptions

From Bond & Harris:

- Negative
  - “a cross between ‘*the European Scream*’ and the reaction of the ostrich”
  - Codification for codification’s sake
  - Too high a cost
- Positive
  - Opinions improved once people had had experience of EC7

### Some Identified Problems

- Is passive pressure a resistance or a favourable action? (Bond & Harris)
- What strength should one use at ULS?
  - Peak? Critical State? (Tony O'Brien, John Atkinson)
- Should water pressures be factored? (Brian Simpson)
- The single source principle – what does it mean? (CIRIA)
- Bond and Harris:
  - “The book deliberately presents ..... a completely different running order from the Eurocodes *so they can be explained more clearly*” (My italics)

### Example of Obscurity

- John Atkinson gave training sessions for Coffey
- He missed out the Model Factor ( $\gamma_{R,d}$ ) for Pile Design by calculation
- After I drew his attention to it, it took him 20 minutes to find the reference in EC7, Even though he knew it must be there

## Consistency with Physical Reality

### Statics Newton's Laws

1. If a body is at rest,  
the sum of the forces acting on it must be zero
2. (Dynamics)
3. To every Action there is always opposed  
an equal Reaction

## ULS Design

- For Limit State EQU:

$$E_{dst;d} \leq E_{stb;d}$$

- For Limit State GEO

$$E_d \leq R_d$$

- How does the inequality affect  
consistency with Newton's Laws?

## ULS Design

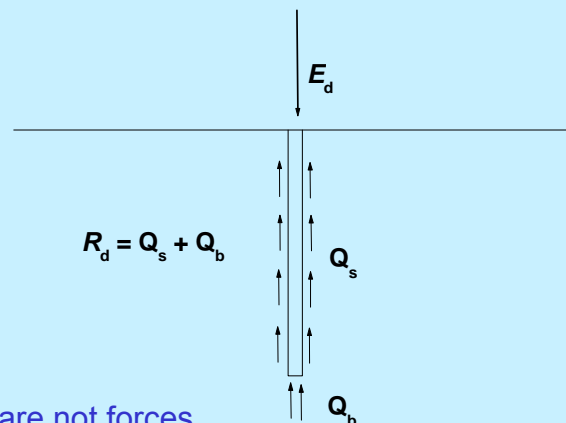
- For Limit State EQU:

$$E_{dst;d} \leq E_{stb;d}$$

- How does the inequality affect consistency with Newton's Laws?

## ULS in Practice

- Pile Design



- The resistances are not forces, they are *capacities* (maximum possible forces)

## ULS in Practice

- Piping with upward water flow
- What is the safety factor?
- It depends on how you define it

- Total Stresses:

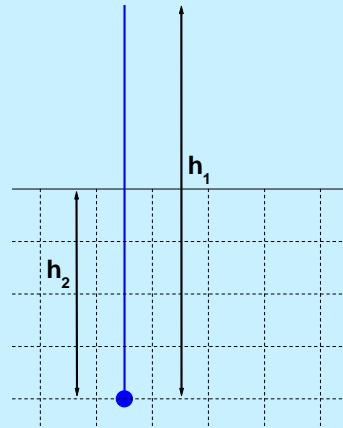
$$- FS = \gamma_b h_2 / \gamma_w h_1$$

- Effective Stresses:

$$- FS = (\gamma_b h_2 - \gamma_w h_2) / (\gamma_w h_1 - \gamma_w h_2)$$

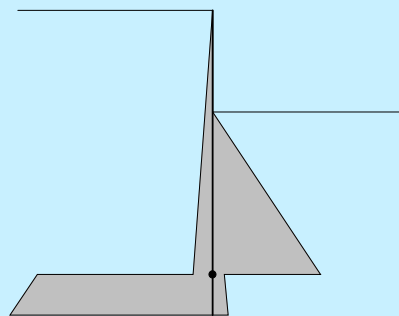
- e.g. For  $h_1 = 3$  m,  $h_2 = 2$  m

- $FS_T = 40/30 = 1.3$        $FS_E = 20/10 = 2$



## ULS in Practice

- Cantilever Wall Design



- Problem of definition of safety factor was addressed during development of CIRIA 104
- We are used to using partial factors for strength
- The resistances depend on the actions  
*and the actions depend upon the resistances*

## Problems with ULS Calculations

- Resistances are sometimes treated as forces, when in fact they are *capacities*
  - i.e resistances and actions interact
- This applies to any ULS calculation, (e.g. Global safety factors) not just EC7 (with partial factors)
  - We are used to this with global safety factors (e.g. Hydraulic uplift, cantilever walls, slopes)
  - Partial factors can make the problem more complex, but not intrinsically different
- A safety factor is still what you define it to be

## Comparison of Global and Partial Safety Factor Methods

- Global safety factors are simpler, and therefore easier to get a feel for
  - For any particular problem and depending on how they are defined
- Partial factors allow better assessment of uncertainty (variation?) of real physical factors (e.g. Variable loadings, material strengths)
  - But how do the code factors relate to real variation? Have they just been chosen to fit previous codes?
- Thinking about partial factors has enabled better identification of inconsistencies in previous practice



## Can we Simplify ULS in EC7?

- Reduce to two sheets of A4:
  
- All except piles & anchors:  
 DA1:C1    A1 + M1 + R1  
 DA1:C2    A2 + M2 + R1
  
- Piles and anchors:  
 DA1:C1    A1 + M1 + R1  
 DA1: C2    A2 + M1 or M2 + R2  
 + Model Factor
  
- And write down what you have done!

Appendix E: EC7 Part 1: Annex A. Table of Partial Factors

| Category       | F.M.R. | Detail | Action      | Partial Factors |               |      |      |      |     |     |     |     |     |      |
|----------------|--------|--------|-------------|-----------------|---------------|------|------|------|-----|-----|-----|-----|-----|------|
| EGU            | =      | G:dst  | Perm        | Destab          | 1.1           |      |      |      |     |     |     |     |     |      |
|                |        |        | Perm        | Slab            | 0.9           |      |      |      |     |     |     |     |     |      |
|                |        |        | Var         | Destab          | 1.5           |      |      |      |     |     |     |     |     |      |
|                |        |        | Var         | Slab            | 0             |      |      |      |     |     |     |     |     |      |
| =              |        |        | f           |                 | 1.25          |      |      |      |     |     |     |     |     |      |
|                |        |        | c           |                 | 1.25          |      |      |      |     |     |     |     |     |      |
|                |        |        | cu          |                 | 1.4           |      |      |      |     |     |     |     |     |      |
|                |        |        | sp          |                 | 1.4           |      |      |      |     |     |     |     |     |      |
|                |        |        | γ           |                 | 1.0           |      |      |      |     |     |     |     |     |      |
| UPL            | =      | G:dst  | Perm        | Destab          | 1.0           |      |      |      |     |     |     |     |     |      |
|                |        |        | Perm        | Slab            | 0.9           |      |      |      |     |     |     |     |     |      |
|                |        |        | Var         | Destab          | 1.5           |      |      |      |     |     |     |     |     |      |
| HYD            | =      | G:dst  |             |                 | 1.55          |      |      |      |     |     |     |     |     |      |
|                |        |        |             |                 | 0.9           |      |      |      |     |     |     |     |     |      |
|                |        |        |             |                 | 1.5           |      |      |      |     |     |     |     |     |      |
| STR + GEO      | =, =   | G      | Perm        | Unf             | 1.35          | 1.0  |      |      |     |     |     |     |     |      |
|                |        |        |             | Fav             | 1.0           | 1.0  |      |      |     |     |     |     |     |      |
|                |        |        |             | Unf             | 1.5           | 1.5  |      |      |     |     |     |     |     |      |
|                |        |        |             | Fav             | 0             | 0    |      |      |     |     |     |     |     |      |
|                |        |        |             |                 |               |      | M1   | M2   |     |     |     |     |     |      |
|                |        |        |             |                 |               |      | 1.0  | 1.25 |     |     |     |     |     |      |
|                |        |        | =           |                 |               |      |      |      |     |     |     |     |     |      |
|                |        |        |             |                 |               |      |      |      |     |     | c   |     | 1.0 | 1.25 |
|                |        |        |             |                 |               |      |      |      |     |     | cu  |     | 1.0 | 1.4  |
|                |        |        |             |                 |               |      |      |      |     |     | sp  |     | 1.0 | 1.4  |
|                |        |        |             |                 |               |      |      |      |     |     | γ   |     | 1.0 | 1.0  |
|                |        |        |             |                 |               |      |      |      |     |     |     |     |     | R1   |
| Shallow        | =      | R v    | Bearing     |                 |               |      |      |      |     |     |     |     |     |      |
|                |        |        | Sliding     | 1.0             | 1.1           | 1.0  |      |      |     |     |     |     |     |      |
| Driven         | =      | b      | Base        |                 | 1.0           | 1.1  | 1.0  | 1.3  |     |     |     |     |     |      |
|                |        |        |             | Shaft           | 1.0           | 1.1  | 1.0  | 1.3  |     |     |     |     |     |      |
|                |        |        |             | Total           | 1.0           | 1.1  | 1.0  | 1.3  |     |     |     |     |     |      |
|                |        |        | s.t         | Shaft tension   |               | 1.25 | 1.15 | 1.1  | 1.6 |     |     |     |     |      |
|                |        |        |             |                 |               |      |      |      |     |     |     |     |     |      |
|                |        |        |             |                 |               |      |      |      |     |     |     |     |     |      |
| Bored          | =      | b      | Base        |                 | 1.25          | 1.1  | 1.0  | 1.6  |     |     |     |     |     |      |
|                |        |        |             | Shaft           | 1.0           | 1.1  | 1.0  | 1.3  |     |     |     |     |     |      |
|                |        |        | t           | Total           |               | 1.15 | 1.1  | 1.0  | 1.5 |     |     |     |     |      |
|                |        |        |             |                 | Shaft tension | 1.25 | 1.15 | 1.1  | 1.6 |     |     |     |     |      |
| CFA            | =      | b      | Base        |                 | 1.1           | 1.1  | 1.0  | 1.45 |     |     |     |     |     |      |
|                |        |        |             | Shaft           | 1.0           | 1.1  | 1.0  | 1.3  |     |     |     |     |     |      |
|                |        |        | t           | Total           |               | 1.1  | 1.1  | 1.0  | 1.4 |     |     |     |     |      |
|                |        |        |             |                 | Shaft tension | 1.25 | 1.15 | 1.1  | 1.6 |     |     |     |     |      |
| Anchors        | =      |        |             |                 |               |      |      |      |     |     |     |     |     |      |
|                |        |        |             |                 |               |      |      |      | 1.1 | 1.1 | 1.0 | 1.1 |     |      |
| Retaining wall | =      | R v    | Bearing     |                 | 1.0           | 1.4  | 1.0  |      |     |     |     |     |     |      |
|                |        |        | Sliding     |                 | 1.0           | 1.1  | 1.0  |      |     |     |     |     |     |      |
|                |        |        | Earth press |                 | 1.0           | 1.4  | 1.0  |      |     |     |     |     |     |      |
| Slopes         | =      | R e    |             |                 | 1.0           | 1.1  | 1.0  |      |     |     |     |     |     |      |

## Conclusions

- Partial Factors constitute the main new feature of EC7 ULS calculations
  
- ULS calculations have intrinsic difficulties
  
- We are used to them in global factor methods, but not yet in partial factor methods
  
- EC7 calculations can be made more straightforward

## Acknowledgements

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  - for Coffey training in EC7  
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